CC010-A



Chelmsford City Council Level 2 Strategic Flood Risk Assessment Detailed Site Summary Tables

Site	Otal	

Site Code	SGS1y
Address	Land between Hoffmans Way and Brook Street (Marriages Mill)
Area	1.53ha
Current land use	Industrial Estate
Proposed land use	Residential
Flood Risk Vulnerability	More Vulnerable

Sources of flood risk

Location of the	site	within
the catchment		

The site is located within the Chelmer Operational Catchment, which is 657.4km². Within the operational catchment, the site is located at the south of the Chelmer (Great Easton - River Can) catchment, which drains 116.2km² of land. This catchment is designated as heavily modified.

Topography

The site is generally flat. The site is approximately 1m lower than the surrounding industrial parks. There are two areas of raised topography within the site: the carpark in the southwest of the site and the industrial unit to the southeast. These areas are raised by approximately 1-2m above the rest of the site. The highest point of the site is in the southeast of the site, at 28.2m AOD, while the lowest section is located in the south at 25.8m AOD. It is important to note that the site is situated within a densely populated and developed urban area, therefore LiDAR data is

It is important to note that the site is situated within a densely populated and developed urban area, therefore LiDAR data is unlikely to be representative of the actual site topography, this may have an impact on some of the flood risk datasets used in this assessment.

Existing drainage features

There are no visible drainage features on the site, however part of the site is previously developed, and is likely to be drained by the existing surface water drainage network. Surface water drains appear to be visible on the site. The River Chelmer flows 25m north of the site.

Critical Drainage Area

The site is not within a critical drainage area.

The proportion of site at risk FMFP:

FZ3 - 0%

FZ2 - 0%

FZ1 - 100%

Fluvial and tidal

The Flood Zone values quoted show the percentage of the site at flood risk from that particular Flood Zone/event, including the percentage of the site at flood risk at a higher risk zone. This is because the values quoted are the area covered by each Flood Zone/extent within the site boundary. For example: Flood Zone 2 includes Flood Zone 3. Flood Zone 1 is the remaining area outside Flood Zone 2 (FZ2+FZ1=100%).

Defended outputs:

3.3% AEP fluvial event - 0%

1% AEP fluvial event - 0%

0.1% AEP fluvial event - 0%

Modelled results show the percentage of site at risk from a given AEP flood event.

Available data:

The proportion of the site at flood risk is determined from the Environment Agency's Flood Map for Planning Flood Zones. This represents the undefended scenario.

Therefore, the defended scenario outputs from the Environment Agency's 1D-2D ISIS-TUFLOW River Chelmer (2018) hydraulic model have been reported as a more accurate representation of the flood risk to this site due to the presence of flood defence structures.

Flood characteristics:

Despite close proximity to the River Chelmer, the site is not expected to experience fluvial or tidal flooding for any of the AEPs modelled (including and up to the 0.1% AEP event). There is thus considered to be a very low hazard posed to the site by fluvial or tidal sources. The closest flood warning and flood alert area is 25m north of the site.

Proportion of site at risk (RoFfSW):

3.3% AEP -30.8%

Max depth - 0.6-0.9m

Max velocity -0.5-1m/s

1% AEP - 42.9%

Max depth - 0.6-0.9m

Max velocity – 1-2m/s

0.1% AEP - 75.7%

Max depth - 0.9-1.2m

Max velocity – 1-2m/s

The % SW extents quoted show the % of the site at surface water risk from that particular event, including the percentage of the site at flood risk at a higher risk zone (e.g. 100-year includes the 30-year %).

The Environment Agency's Risk of Flooding from Surface Water mapping was used in this assessment.

Description of surface water flow paths:

During the 3.3% AEP surface water event, floodwater is present on the site from the north along its boundary with Hoffmanns Way and along the southern border with SGS1v. Water accumulates in lower lying sections of the centre and south of the site, reaching a maximum depth and velocity of 0.9m and 1m/s, respectively. This event generates a maximum hazard of "Danger for most – includes the general public".

Under the 1% AEP event, much of the southern portion of the site is expected to become inundated with up to 0.9m water travelling at up to 1 m/s. This appears to enter the site along the southern

Surface Water

	boundary with SGS1v. Similar areas are expected to become inundated to the 3.33% AEP event, albeit to greater depths and velocities. The maximum depth and velocity on the site are quoted above, and the hazard rating is, similar to the previous model, designated as "Danger for most".
	In the 0.1% AEP event, almost all areas of the site are inundated with surface water. The maximum depth, hazard, and velocity is listed above. The greatest depths under this event are found in the southern and central sections of the site, and appears to be entering via the boundary with SGS1v and Hoffmanns Way. Here, a maximum hazard of "Danger for most" is expected, albeit for a much greater proportion of the site than the other simulated events.
	It is important to note that the accuracy of the EA LiDAR Digital Terrain Model (DTM), used in this surface water flood model, appears to be limited by the urban nature of the site, introducing uncertainty. The risk from surface water to the site will need to be quantified as part of a site-specific flood risk assessment.
Reservoir	The site is not expected to be at risk from reservoir flooding under either a dry or wet day scenario.
	The JBAs Groundwater Emergence Map, is provided as 5m resolution grid squares.
Groundwater	The southwest of the site is shown to be at the greatest risk of groundwater emergence, as groundwater levels are either at or very near (within 0.025m of) the ground surface. As a result, there is a risk of groundwater flooding to both surface and subsurface assets. Groundwater may emerge at significant rates and has the capacity to flow overland and/or pond within any topographic low spots. The majority of the site has groundwater levels between 0.025m and 0.5m below the ground surface. Within this zone there is a risk of groundwater flooding to both surface and subsurface assets. There is the possibility of groundwater emerging at the surface locally. The eastern perimeter of the site is shown to have negligible risk of groundwater flooding, and any groundwater flooding incidence has a chance of less than 1% annual probability of occurrence.
	This will need to be assessed further as part of a site-specific flood risk assessment and is likely to require ground investigations to determine the true risk to the site.
Sewers	Sewer flooding records were not available for this assessment. The entirety of Chelmsford is identified as a Flood priority catchment in Anglian Water's Drainage and Wastewater Management Plan (DWMP). Developers should consult Anglian Water as part of any development proposal to ensure development does not exacerbate existing issues and maximise opportunities for development to deliver benefits in line with the long term strategic aims set out in the DWMP.
	The Environment Agency's Historic Flood Map shows no records of flooding on the site, with the closest area 25m north of the site, associated with the River Chelmer.
Flood history	Essex County Council as LLFA has one record of flooding near the site, but the source and date of flooding is unknown. There are no other flood records within a 500m radius of the site.
Flood risk management infrastructure	

Defences	The Environment Agency (EA) AIMS dataset shows that the site is not protected by formal flood defences along the River Chelmer. The southern perimeter of the site is classed by the EA as natural high ground, with a recorded standard of protection of 100 years.
Residual risk	There are no formal flood defences within the vicinity of the site, therefore there is no residual risk of defence failure. The natural high ground along the River Chelmer is recorded to protect to a 1% AEP flood event, although modelling suggests the standard of protection is higher. The most recent Visual Asset Inspection (05 April 2023) found that the natural high ground protecting the site was in good condition.
	The Margaretting Flood Alleviation Scheme to safeguard the city centre was cancelled in March 2022. The risk from flooding remains. The City Council continues to work with the Environment Agency to supplement existing flood defences and deliver a new series of catchment-based measures under the Chelmsford Flood Resilience Partnership. Sites affected by flood risk should devise an FRA on the basis that existing city centre flood defences are in place and, if sufficiently advanced, the catchment-based measures identified by the Chelmsford Flood Resilience Partnership project. In either scenario a financial contribution to the Chelmsford Flood Resilience Partnership project would be required.
	Whilst there are currently no formal defences within the vicinity of the site, developers should consult with Chelmsford City Council and the Environment Agency to identify whether land within the site boundary may need to be safeguarded for flood defences in future. If defences are proposed as part of the development, maintenance arrangements (including funding mechanisms) for the defences will need to be demonstrated for the lifetime of development.
Emergency planning	
Flood warning	The site is not located within a Flood Warning or Flood Alert area. The nearest Flood Warning/Flood Alert area is 25m to the north of the site.
	Access and egress to the site for vehicles and pedestrians is currently possible to the north via an access road connected to Hoffmanns Way, and along Brook Street to the southwest of the site.
	Access to the site is expected to remain possible for all modelled fluvial AEP events including climate change scenarios.
Access and egress	Under a 3.33% AEP surface water flood event, depths of up to 0.6m are expected at the access road's junction with Hoffmanns Way, travelling at up to 2 m/s. This presents a hazard to most, including the general public. It is assumed that safe access and egress is not possible under this scenario. The southwestern access point via Brook Street is expected to be inundated to depths of up to 0.6m and velocities of 0.5m/s. This represents a danger to some, including children, the elderly and the infirm.
	Under a 1% AEP surface water flood event, water depths of up to 0.9m and velocities of up to 2.0m/s are expected along Hoffmann's Way. Flood depths along Brook Street are up to 0.6m, travelling at up to 1m/s. For both access points, this corresponds to a hazard to most, including the general public.
	Under a 0.1% AEP surface water flood event, depths of up to 0.9m and velocities of over 2 m/s are expected along both Hoffmann's

Way and Brook Street. For both access points, this corresponds to a hazard of 'Danger for most'.

While the design surface water flood event (1% AEP+40% Climate Change - CC), similar depths, velocities and hazards are expected, for a greater extent than the non-CC scenario.

Arrangements for safe access and egress will need to be demonstrated for the design event (1% AEP plus 40% CC), using the depth, velocity, and hazard outputs. Any raising of access routes should not impede surface water flow routes, or increase flood risk elsewhere. If safe access and egress cannot be demonstrated, a Flood Warning and Evacuation Plan should be prepared for the site.

Dry Islands

The site is not located on a dry island.

Climate change

Management Catchment: Combined Essex Management Catchment

Increased storm intensities due to climate change may increase the extent, depth, velocity, hazard, and frequency of both fluvial and surface water flooding.

Fluvial

The River Chelmer has available climate change outputs for the Central (25%) and Upper End (72%) allowances for the 2080s, with the former representing the design event.

Under all available climate change simulations, the site remained dry for all AEPs, and thus fluvial flooding is expected to remain a very low hazard to the site, even with climate change.

Surface Water:

Implications for the site

The latest climate change allowances have also been applied to the Risk of Flooding from Surface Water map to indicate the impact on surface water flood risk. The 1% AEP plus 40% climate change corresponds to the 1% AEP upper end allowance for peak rainfall intensity for the 2070s epoch and is therefore the 'design event' scenario.

Under the design event surface water flood, previously unflooded sections of the access road and building footprints are expected to become inundated, at a far greater spatial extent than the standard 1% AEP. Under this scenario, 72.9% of the site is expected to be affected by flooding, up from 42.9% under the 1% AEP scenario, suggesting the area is relatively sensitive to climate change. Furthermore, maximum flood depth is expected to increase from 0.9m to 1.1m under the design event compared to the current 1% AEP.

Development proposals at the site must address the potential changes associated with climate change and be designed to be safe for the intended lifetime. The provisions for safe access and egress must also address the potential increase in severity and frequency of flooding.

Requirements for drainage control and impact mitigation

Broad-scale assessment of possible SuDS

Geology & Soils

Geology at the site consists of:

- Bedrock Geology London Clay Formation consisting of clay, silt and sand.
- Superficial Geology The site is expected to have sedimentary River Terrace Deposits of sand and gravel. The eastern perimeter of the site is likely to consist of alluvial deposits of clay, silt, sand and gravel.
- Soils at the site consist of:
 - Freely draining slightly acid loamy soils.

SuDS

- Groundwater levels are indicated to be less than 0.5m below ground level during a 1% AEP event. Detention and attenuation features should be designed to prevent groundwater ingress from impacting hydraulic capacity and structural integrity. Additional site investigation work may be required to support the detailed design of the drainage system. This may include groundwater monitoring to demonstrate that a sufficient unsaturated zone has been provided above the highest occurring groundwater level. Below ground development such as basements are not appropriate at this site.
- BGS data indicates that the underlying geology is clay, silt and sand which is likely to be with highly variable permeability. This should be confirmed through infiltration testing. Off-site discharge in accordance with the SuDS hierarchy may be required to discharge surface water runoff from the site.
- The site is not located within a Groundwater Source Protection Zone and there are no restrictions over the use of infiltration techniques with regard to groundwater quality.
- The site is not located within a historic landfill site.
- The site is designated in two Nitrate Vulnerable Zones (NVZs)
 - Surface Water "Surface Water S428 River Chelmer NVZ"
 - Groundwater "Groundwater G78 Sandlings and Chelmsford"
- The site is also within a Drinking Water Safeguard Zone (SWSGZ1029), meaning it is at risk from nutrients and certain pesticides.
- Surface water discharge rates should not exceed predevelopment discharge rates for the site and should be designed to be as close to greenfield runoff rates as reasonably practical in consultation with the LLFA. It may be possible to reduce site runoff by maximising the permeable surfaces on site using a combination of permeable surfacing and soft landscaping techniques.
- The Risk of Flooding from Surface Water (RoFSW) mapping indicates the presence of surface water flow paths during the 3.33% AEP event. Existing flow paths should be retained and integrated with blue-green infrastructure and public open space.
- If it is proposed to discharge runoff to a watercourse or sewer system, the condition and capacity of the receiving watercourse or asset should be confirmed through surveys and the discharge rate agreed with the asset owner.

Opportunities for wider sustainability benefits and integrated flood risk management

- Implementation of SuDS at the site could provide opportunities to deliver multiple benefits including volume control, water quality, amenity and biodiversity. This could provide wider sustainability benefits to the site and surrounding area. Proposals to use SuDS techniques should be discussed with relevant stakeholders (LPA, LLFA and EA) at an early stage to understand possible constraints.
- Development at this site should not increase flood risk either on or off site. The design of the surface water management proposals should take into account the impacts of future climate change over the projected lifetime of the development.
- Opportunities to incorporate filtration techniques such as filter strips, filter drains and bioretention areas must be considered. Consideration should be made to the existing condition of receiving waterbodies and their Water Framework Directive objectives for water quality. The use of multistage SuDS treatment will clean and improve water quality of surface water runoff discharged from the site and reduce the impact on receiving water bodies.
- Opportunities to incorporate source control techniques such as green roofs, permeable surfaces and rainwater harvesting must be considered in the design of the site.
- The potential to utilise conveyance features such as swales to intercept and convey surface water runoff should be considered. Conveyance features should be located on common land or public open space to facilitate ease of access. Where slopes are >5%, features should follow contours or utilise check dams to slow flows.

NPPF and planning implications

Exception Test requirements

The site is at significant risk from surface water flooding and groundwater flooding. Whilst the Exception Test is only required for sites at risk from fluvial flooding, it is recommended that Chelmsford City Council carefully weigh up the benefits of developing the site against the significant surface water flood risk. Developers will need to demonstrate through a site-specific flood risk assessment that users of the site will be safe throughout its lifetime.

•

A site-specific Flood Risk Assessment (FRA) is required due to the risk of surface water and groundwater flooding and the proposed development constituting a change of use to a more vulnerable class (industrial to residential).

Flood Risk Assessment:

- All sources of flooding should be considered as part of a sitespecific FRA.
- Consultation with Chelmsford City Council, Essex County Council, Anglian Water, and the Environment Agency should be undertaken at an early stage.
- Any FRA should be carried out in line with the National Planning Policy Framework (NPPF); Flood Risk and Coastal Change Planning Practice Guidance (PPG); and the Council's Local Plan Policy's and SuDS Strategy.
- The development should be designed with mitigation measures in place where required.
- Ground investigations will be necessary to confirm groundwater risk. This is also likely to impact upon the types of SuDS that are suitable for the site.

Guidance for site design and making development safe:

Requirements and guidance for site-specific Flood Risk Assessment

- The developer will need to show, through an FRA, that future users of the development will not be placed in danger from flood hazards throughout its lifetime. It is for the applicant to show that the development meets the objectives of the NPPF's policy on flood risk. For example, how the operation of any mitigation measures can be safeguarded and maintained effectively through the lifetime of the development. (Para 048 Flood Risk and Coastal Change PPG).
- The risk from surface water flow routes should be quantified as part of a site-specific FRA, including a drainage strategy, so runoff magnitudes from the development are not increased by development across any ephemeral surface water flow routes. A drainage strategy should help inform site layout and design to ensure runoff rates are as close as possible to predevelopment greenfield rates.
- Arrangements for safe access and egress will need to be provided for the 1% AEP fluvial and rainfall events with an appropriate allowance for climate change, considering depth, velocity, and hazard. Design and access arrangements will need to incorporate measures, so development and occupants are safe. If safe access/egress cannot be provided in the design event, a Flood Warning and evacuation Plan should be prepared if the site is bought forwards.
- Provisions for safe access and egress should not impact on surface water flow routes or contribute to loss of floodplain storage. Consideration should be given to the siting of access points with respect to areas of surface water flood risk.
- Flood resilience and resistance measures should be implemented where appropriate during the construction phase, e.g. raising of floor levels and use of boundary walls. These measures should be assessed to make sure that flooding is not increased elsewhere.

Key messages

The majority of the site faces a considerable risk of either surface water, groundwater flooding, or both, and is sensitive to climate change impacts. Therefore, careful consideration will need to be given to these issues if the site is to be brought forward. Whilst the Exception Test is only explicitly required for sites at fluvial risk, careful consideration should be giving to these issues before the site development can is brought forwards. Chelmsford City Council should satisfy themselves that the benefits of development outweigh the risk. With regards to the flood risk, development may be able to proceed if:

- Safe access and egress can be demonstrated in the surface water 1% AEP and 1% AEP plus climate change events. This includes measures to reduce flood risk along these routes such as raising access, but not displacing floodwater elsewhere. If safe access and egress cannot be provided, an adequate flood warning and evacuation plan should be prepared.
- A carefully considered and integrated flood resilient and sustainable drainage design is put forward, with development steered away from the areas identified to be at risk of surface water flooding across the site, such as the centre and southern border.
- Ground investigations will be necessary to confirm groundwater risk. This is also likely to impact upon the types of SuDS that are suitable for the site.
- A site-specific FRA demonstrates site users will be safe throughout the lifetime of the development and that development of the site does not increase the risk of surface water flooding on the site and to neighbouring areas.
- If flood mitigation measures are implemented then they are tested to check that they will not displace water elsewhere (for example, if land is raised to permit development on one area, compensatory flood storage will be required in another).

Mapping Information

The key datasets used to make planning recommendations for this site were the Environment Agency's Flood Map for Planning, the Environment Agency's Risk of Flooding from Surface Water map and the Environment Agency's River Chelmer 2018 model. More details regarding data used for this assessment can be found below.

Flood Zones	Flood Zones 2 and 3 have been taken from the Environment Agency's Flood Map for Planning mapping.
Climate change	The River Chelmer (2018) Environment Agency model has been used in this assessment.
Fluvial and tidal extents, depth, velocity and hazard mapping	The central and upper end allowances were available for the River Chelmer (2018) hydraulic model to indicate the impacts on fluvial flood risk.
Surface Water	The latest climate change allowances (updated May 2022) have also been applied to the Risk of Flooding from Surface Water map to indicate the impact on pluvial flood risk.
Surface water depth, velocity and hazard mapping	The surface water depth, velocity, and hazard mapping for the 3.3%, 1% and 0.1% AEP events (considered to be high, medium, and low risk) have been taken from Environment Agency's RoFSW.